

**TEMPORARY STREET LIGHT POLE**

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**Project: EZ SLIP BARRIER CONNECTION BRACKET**

**General Data:**

C.G of Fixture **22 ft**  
 Base Height Above Ground **3.5 ft**  
 C.G. of Fixture from Pole CL **1.5 ft**  
 EPA of Fixture (total) **2.3 sf**  
 Weight of Fixture (total) **75 lbs**  
 Wind Speed (V) **90 mph**  
 I r = **1**  
 Material for Pole & Arm **A500**  
**Pole Data:**  
 Shape of Pole **Rd**  
 Length **20 ft**  
 Base OD **2.875 in**  
 Top OD **2.875 in**  
 Wall Thickness **0.276 in**  
 Aver. Unit Wt /LF **7.67 lbs/lf**  
 Area Aver. **2.25 in^2**  
 r = **1.30 in**  
 Area @ Base **2.25 in**  
 Section Modulus @ Base **1.34 in^3**  
 Mom. of Inertia @ Base **1.92 in^4**  
 Torsion Constant (J) **2.93 in^4**  
 Projected Area of Pole **4.79 sf**  
 Exposure category **C**  
 Kz for Pole: **0.95**  
 Vd = **21.5625** Vd<39 so  
 Cd for Pole (based on wind tunnel) **1.00**  
 Gust Factor G = **1.14**  
 Overload factor = **1.33**

**Calculations: AASHTO Wind Calc**

Wind Pressure =  $0.00256 \cdot V^2 \cdot Kz \cdot G \cdot Ir$   
 $P = 22.44 \cdot *Cd, psf$  P fat = **1.6 psf**  
 (conservative)

**Fixtures:** Design Wind Fatigue Wind @ 25 mph  
 Dead Load **75 lbs** **75**  
 Wind Load **51.61 lbs** **3.68**  
 OT Moment **1316.02 ft-lbs** **93.84**

**Pole:**

Dead Load: **153.31 lbs** **153.31**  
 EPA **4.77 sf** **5.27**  
 Wind Load: **107.09 lbs** **8.43**  
 C.G. of Pole **10.00 ft** **10.00**  
 OT Moment **1070.88 ft-lbs** **84.33**  
 Torsion M **77.41 ft-lbs** **5.52**

**Sum of Loads @ Base:**

axial **228.31 lbs** **228.31**  
 shear **158.70 lbs** **12.11**  
 moment **2386.90 ft-lbs** **178.17**  
 torsion **77.41 ft-lbs** **5.52**

Material Yield = **42000 psi**

**Allowable Stresses: Welded - no heat treat**

Fa =  $.6 \cdot Fy =$  **25,200 psi**  
 Fb =  $.66 \cdot Fy =$  **27,720 psi**  
 Fv =  $.33 \cdot Fy =$  **13,860 psi**  
 F fatigue = **2600 psi**

**Stress Calculations:**

	Design Wind	Fatigue Wind
Pole: fa = axial / Area =	101.4 psi	101.4
fb = OT Mom / Sect Mod. =	21375.2 psi	1595.6
fv = $(2 \cdot \text{shear}/a) + (\text{torsion}/j) =$	458.3 psi	33.4

**CSR's:**

Design Wind:	Fatigue Wind:
Post: CSR = $fa/Fa + (fb/Fb) + (fv/Fv)^2 =$ <b>0.58</b>	CSR = $fa/Fa + (fb/Fb) + (fv/Fv)^2 =$ <b>0.67</b>
<b>csr &lt; 1.0 so OK!</b>	<b>csr &lt; 1.0 so OK!</b>

**NOTE: By inspection, since pole diameter and wall thickness is less that base stub pole diameter and wall thickness, stub tube design is OK.**

Maximum Bending in Base Plate:	A36 Plate	Maximum Bending in Slip Plates
Minimum Width	4.75 in	1 in
Thickness	0.5 in	2.5 in
Min dist betwn plates	6.125 in	2.375 in
initial Mom of Inertia =	0.049479 in^4	1.3020833 in^4
Area =	2.375 in^2	2.5 in^2
Transformed Moment =	44.5498 in^4	16.425781 in^4
Sect Modulus =	14.547 in^3	13.832 in^3
DesignMoment =	2386.90 ft-lbs	2386.90 ft-lbs
Design Stress =	1968.996 psi	2070.724 psi
Allowable Stress =	35910 psi so OK	35910 psi so OK
Fatigue Moment =	178.17 ft-lbs	178.17 ft-lbs
Fatigue Stress =	146.9786 psi	154.57225 psi
Allowable Stress =	2600 psi so OK	2600 psi so OK

